1'-3 1/2"

1'-3 1/2"

Face of Abut Bkwl-

5'-0"

1'-0"

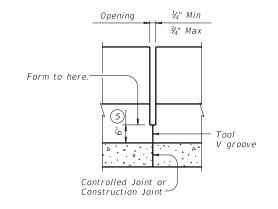
(HSS rail not shown for clarity)

2'-0"

Wingwall Length (Variable) 5'-0" Min

 $\frac{B}{B}$

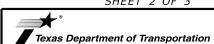
(2)



POST JOINT DETAIL

(Showing without raised sidewalk) Provide at all interior bents without slab expansion joints. Location independent of HSS rail splices.

SHEET 2 OF 3



Bridge Division Standard

COMBINATION RAIL

TYPE C223

CTXDOT July 2014 CONT SECT JOB HIGHWAY

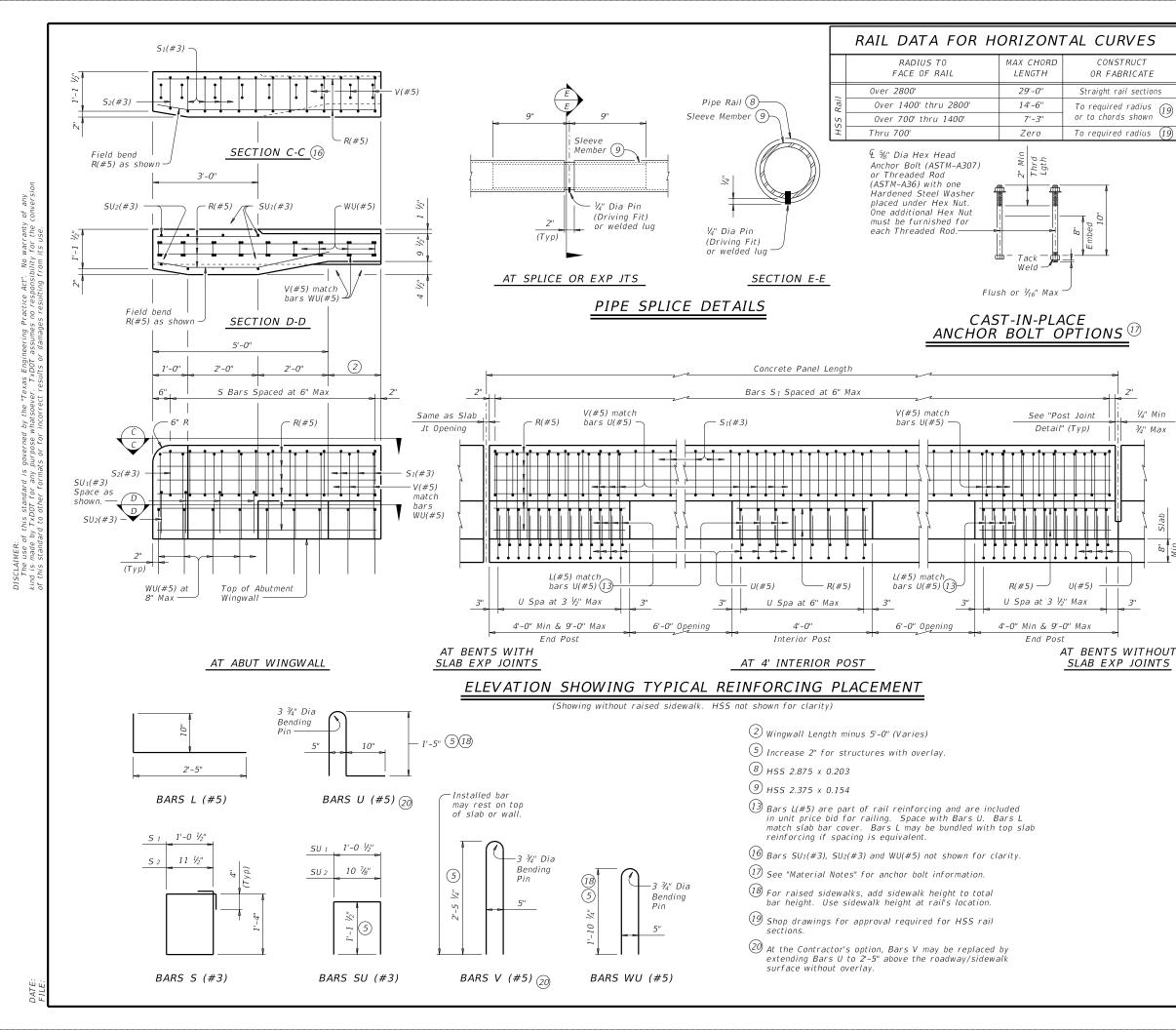
DIST COUNTY SHEET NO.

1'-3 1/2"

1'-3 1/2"

12 Top longitudinal slab bar may be adjusted laterally 3" plus or

minus to tie reinforcing.



CONSTRUCTION NOTES:

Face of rail, posts and parapet must be vertical transversely unless otherwise approved by the Engineer. HSS rail posts and opening end faces must be perpendicular to top of adjacent concrete parapet grade. Use epoxy mortar under HSS rail post base plates if gaps larger than 1/16" exist.

Provide water barriers at openings draining onto undercrossing roadways and sidewalks. They may be cast-in-place or precast in convenient lengths and bonded to the bridge deck with an approved enoxy cement.

HSS rail sections must not include less than two posts, and no more than four (except at Abutments).

Round or chamfer exposed edges of HSS rail and HSS rail posts to approximately 1/16" by grinding.

At the Contractor's option anchor bolts may be cast with the parapet (See Cast-in-Place Anchor Bolt Options)

MATERIAL NOTES:

1/4" Min

Galvanize all steel components except reinforcing unless otherwise shown on plans.

Provide Class "C" concrete. Provide Class "C" (HPC) if required elsewhere. Chamfer all exposed corners.

Epoxy coat all rail reinforcement if slab bars are epoxy coated. Provide Grade 60 reinforcing steel.

Anchor bolts must be 3/8" Dia ASTM A36 threaded rods with one hex nut and one hardened steel washer at each bolt. Embed threaded rods into parapet wall with a Type III Class C epoxy anchorage system. Minimum embedment depth is 3". Anchorage system chosen must be able to achieve an ultimate tensile resistance of 8.4 kips per bolt. The Contractor must provide evidence to the Engineer that this can be achieved. Evidence of adequate tensile resistance can be based on the Manufacturer's published values of ultimate tensile strength (account for anchor spacing and edge distance). Anchor installation, including hole size, drilling, and clean-out, must be in accordance with the Manufacturer's instructions.

Optional cast-in-place anchor bolts must be 5%" Dia ASTM A307 Grade A bolts (or A36 threaded rods with one tack welded hex nut each) with one hex nut and one hardened steel washer at each bolt. Provide ASTM-A1085, A500 Grade B or A53 Grade B for all HSS.

Deformed Welded Wire Reinforcing (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars U, V, and WU unless noted otherwise.

Provide bar laps, where required, as follows:

Uncoated $\sim #5 = 1'-9''$ Epoxy coated ~ #5 = 2'-7"

Bridge Division Standard

GENERAL NOTES:

This rail has been evaluated and accepted to be of equal strength to railings with like geometry, which have been crash tested to meet NCHRP Report 350 TL-4 criteria. However, its use is limited to speeds of 45 mph or less due to the presence of the HSS rail. Do not use this railing on bridges with expansion joints providing

more than 5" movement. Rail anchorage details shown on this standard may require modification for select structure types. See appropriate details

elsewhere in plans for these modifications. Submit erection drawings showing panel lengths, HSS rail post spacing, and anchor bolt setting to the Engineer for approval.

Average weight of railing with no overlay

370 plf total 358 plf (Conc) 12 plf (Steel)

Cover dimensions are clear dimensions, unless noted otherwise.

Reinforcing bar dimensions shown are out-to-out

SHEET 3 OF 3



COMBINATION RAIL

TYPE C223

FILE: 1	Istd019.dgn	DN: TXDOT		ck: TxD0T	DW:	JTR		ск: ЈМН
©T x D0T	July 2014	CONT	SECT	JOB		HIGHWAY		
	REVISIONS							
		DIST	COUNTY			SHEET NO.		